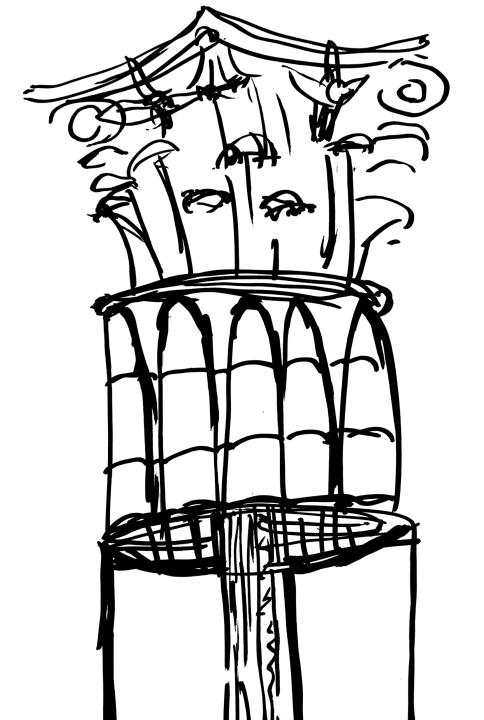
# Combined Stress 4/19

HW – Combined Stress

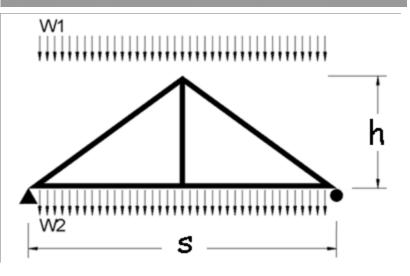
Lab - Combined Stress

Structure II Section 004

Yifan Ma yifanma@umich.edu



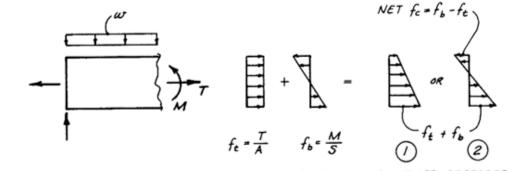
DATASET: 1 -23-	
Full span of truss	13 FT
Height of truss	5 FT
On Center spacing of trusses	24 IN
Size of bottom chord	2x8
Actual width, b	1.5 IN
Actual depth, d	7.25 IN
Snow Load on roof, w1	30 PSF
Live Load in attic, w2	35 PSF
Factored allowable bending stress, F'b	1064 PSI
Factored allowable tension stress, F't	633 PSI



# **HW - Combined Stress**

Data: geometry, load

Required: pass or fail



- 1.Determine truss joint loading
- 2.Determine the external end reactions of the whole truss.
- 3.Use an FBD of the reaction joint to find the chord forces. Sum the forces horizontal and vertical to find the components.
- 4. Calculate the actual axial and flexural stress.
- 5.Determine allowable stresses using applicable factors
- 6.Check NDS equations

#### 3.9.1 Bending and Axial Tension

Members subjected to a combination of bending and axial tension (see Figure 3G) shall be so proportioned that:

$$\frac{f_t}{f_t} + \frac{f_b}{f_b^*} \le 1.0$$
 TENSION CRIT. (3.9-1)

and

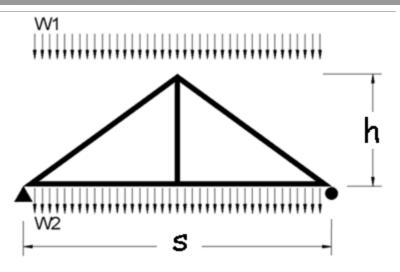
$$\frac{f_b - f_t}{F_b} \le 1.0$$
 FLEXURE CRIT. (3.9-2)

#### where:

F<sub>b</sub> = reference bending design value multiplied by all applicable adjustment factors except C<sub>L</sub>

F<sub>b</sub>" = reference bending design value multiplied by all applicable adjustment factors except C<sub>v</sub>

DATASET: 1 -23-	
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## 1. Load on one truss-top chord, w1

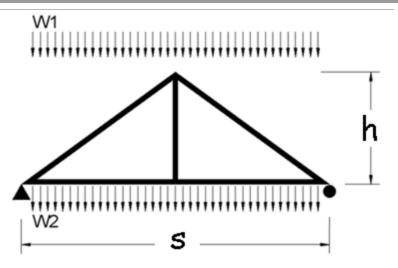
$$w1_{top} = w1(area) * o.c.spacing = 30*24/12 = 60 plf$$

#### 2. Load on one truss-bottom chord, w2

$$w2_bottom = w2(area) * o.c. spacing = 35*24/12 = 70 plf$$

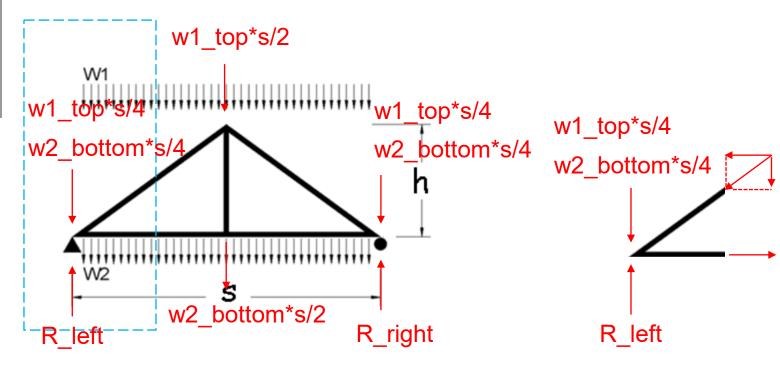
### 3. Total left reaction due to total load (w1 and w2)

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Full span of truss	13 FT
Height of truss	5 FT
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Size of bottom chord	2x8
Actual width, b	1.5 IN
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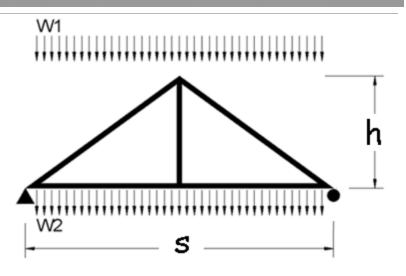


### 4. Vertical force component in truss top chord (no sign)

$$\sum Fv = Fv_{top} + w1_{top}*s/4 + w2_{bottom}*s/4 - R_{left} = 0$$



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Factored allowable bending stress, F'b	1064 PSI
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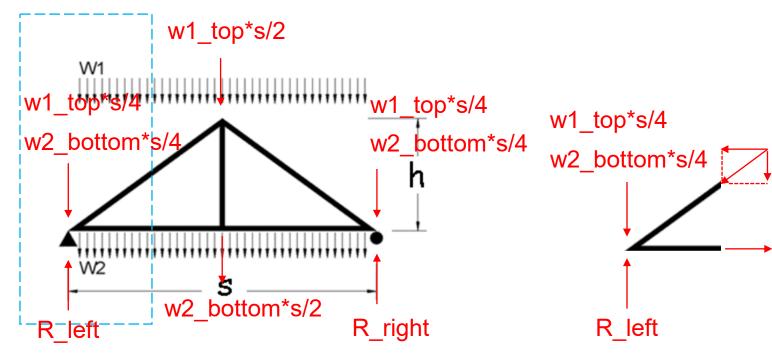
### 5. Horizontal force component in truss top chord(no sign)

$$\frac{Fv\_top}{Fh\_top} = \frac{h}{s/2}$$

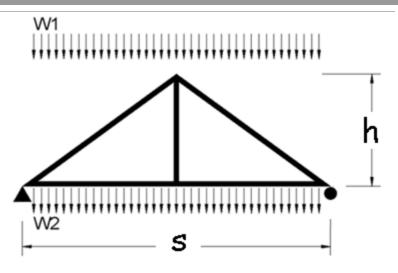
Fh\_top =(
$$Fv_top * s/2$$
) /h = (422.5\*13/2) / 5 = 549.25 lbs

### 6. Axial force in the truss bottom chord(- if Compression)

$$\Sigma$$
Fh =Fh\_bottom-Fh\_top = 0



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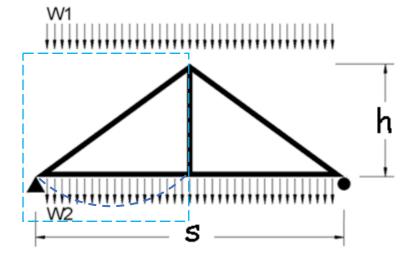
#### 7. Area of the bottom chord member

Area\_bottom = 
$$b \times d = 1.5*7.25 = 10.875 \text{ in}^2$$

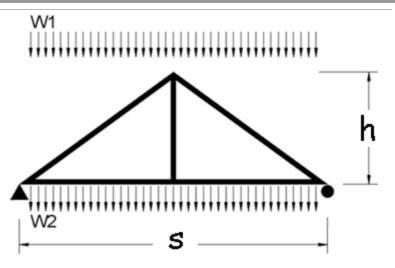
## 8. Axial stress in the bottom chord(-Compression)

### 9. Maximum bending moment in the bottom chord member

$$M = wl^2/8 = w2*(s/2)^2/8 = 70*(13/2)^2/8 = 369.69 \text{ ft-lbs}$$



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#### 10. Section modulus of the bottom chord member Sx

NDS supplement Table 1B

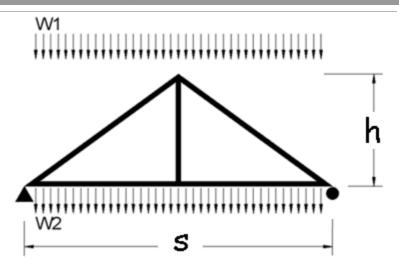
 $Sx = 13.14 \text{ in}^3$ 

			X->	( AXIS	Y-1	/ AXIS						
Nominal	Standard Dressed	Area of	Section	Moment of	Section	Moment of	Approximate weight in pounds per linear foot of piece when density of wood equa					
Size b x d	Size (S4S) b x d in. x in.	Section A in. <sup>2</sup>	Modulus S <sub>xx</sub> in. <sup>3</sup>	Inertia I <sub>xx</sub> in. <sup>4</sup>	Modulus S <sub>yy</sub> in. <sup>3</sup>	Inertia I <sub>yy</sub> in. <sup>4</sup>	25 lbs/ft <sup>3</sup>	30 lbs/ft <sup>3</sup>	35 lbs/ft <sup>3</sup>	40 lbs/ft <sup>3</sup>	45 lbs/ft <sup>3</sup>	50 lbs/ft <sup>3</sup>
Boards <sup>1</sup>												
1 x 3	3/4 x 2-1/2	1.875	0.781	0.977	0.234	0.088	0.326	0.391	0.456	0.521	0.586	0.651
1 x 4	3/4 x 3-1/2	2.625	1.531	2.680	0.328	0.123	0.456	0.547	0.638	0.729	0.820	0.911
1 x 6	3/4 x 5-1/2	4.125	3.781	10.40	0.516	0.193	0.716	0.859	1.003	1.146	1.289	1.432
1 x 8	3/4 x 7-1/4	5.438	6.570	23.82	0.680	0.255	0.944	1.133	1.322	1.510	1.699	1.888
1 x 10	3/4 x 9-1/4	6.938	10.70	49.47	0.867	0.325	1.204	1.445	1.686	1.927	2.168	2.409
1 x 12	3/4 x 11-1/4	8.438	15.82	88.99	1.055	0.396	1.465	1.758	2.051	2.344	2.637	2.930
	n Lumber (see N				NDS 4.1.3							
2 x 3	1-1/2 x 2-1/2	3.750	1.56	1.953	0.938	0.703	0.651	0.781	0.911	1.042	1.172	1.302
2 x 4	1-1/2 x 3-1/2	5.250	3.06	5.359	1.313	0.984	0.911	1.094	1.276	1.458	1.641	1.823
2 x 5	1-1/2 x 4-1/2	6.750	5.06	11.39	1.688	1.266	1.172	1.406	1.641	1.875	2.109	2.344
2 x 6	1-1/2 x 5-1/2	8.250	7.56	20.80	2.063	1.547	1.432	1.719	2.005	2.292	2.578	2.865
2 x 8	1-1/2 x 7-1/4	10.88	13.14	47.63	2.719	2.039	1.888	2.266	2.643	3.021	3.398	3.776
2 x 10	1-1/2 x 9-1/4	13.88	21.39	98.93	3.469	2.602	2.409	2.891	3.372	3.854	4.336	4.818
2 x 12	1-1/2 x 11-1/4	16.88	31.64	178.0	4.219	3.164	2.930	3.516	4.102	4.688	5.273	5.859
2 x 14	1-1/2 x 13-1/4	19.88	43.89	290.8	4.969	3.727	3.451	4.141	4.831	5.521	6.211	6.901

### 11. Maximum bending stress in the bottom chord member

fb = M/Sx = 369.69\*12/13.14 = 337.62 PSI

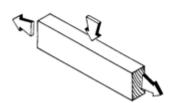
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Factored allowable tension stress, F't	633 PSI



## 12. Combined stress using NSD equation 3.9-1

$$ft/F't + fb/F'b = 50.51/633+337.62/1064$$
  
= 0.397 < 1

#### Figure 3G Combined Bending and Axial Tension



### 13.Combined stress using NSD equation 3.9-2

$$(fb-ft)/F'b = (337.62-50.51)/1064$$
  
= 0.270 < 1

### 14. Does member pass?

Pass,1

#### 3.9.1 Bending and Axial Tension

Members subjected to a combination of bending and axial tension (see Figure 3G) shall be so proportioned that:

$$\frac{f_t}{F_t} + \frac{f_b}{F_b^*} \le 1.0$$
 TENSION CRIT. (3.9-1)

and

$$\frac{f_b - f_t}{F_b^{"}} \le 1.0$$
 FLEXURE CRIT. (3.9-2)

#### where:

F<sub>b</sub> = reference bending design value multiplied by all applicable adjustment factors except C<sub>L</sub>

F<sub>b</sub>" = reference bending design value multiplied by all applicable adjustment factors except C<sub>v</sub>

# LAB – Combined Stress

## **Description**

This project uses observation of a physical trial to see the effects of flexure combined with tension or compression.

#### Goals

To observe the behavior of tension + flexure
To observe the behavior of compression +
flexure

To estimate the addition of combined stress profiles

To observe the results of P + delta loading

#### Procedure

- Load the 12 inch wood stick with 4 washers at midspan as shown below. The stick is 1/16"x1/2" A=0.03125 in<sup>2</sup> Sy=0.0003255 in<sup>3</sup> 4 washers = 0.15 lbs.
- Note the deflection caused by the load. Calculate the flexure stress.
- Next apply an additional axial tension force to the stick of approximately 10 lb (pull on it) and note the change in deflection. Calculate the additional axial stress.
- Make a sketch showing the addition of the stress profiles of flexure + tension.
- Now apply (or try) an axial compression load of approximately 10 lb to the stick and again note the change in deflection. Again calculate the axial stress.
- Make a sketch showing the addition of the stress profiles of flexure + compression.
- 7. What additional load and stress is being neglected in the case of compression + flexure?



$$M = \frac{P\,L}{4} \qquad f_b = \frac{M}{S_y} \qquad f_t = \frac{P}{A} \qquad f_c = \frac{P}{A} \qquad f_{comb} = \pm \frac{M}{S_y} \pm \frac{P}{A}$$

- Load the 12 inch wood stick with 4 washers at midspan as shown below. The stick is 1/16"x1/2" A=0.03125 in<sup>2</sup> Sy=0.0003255 in<sup>3</sup> 4 washers = 0.15 lbs.
- Note the deflection caused by the load. Calculate the flexure stress.

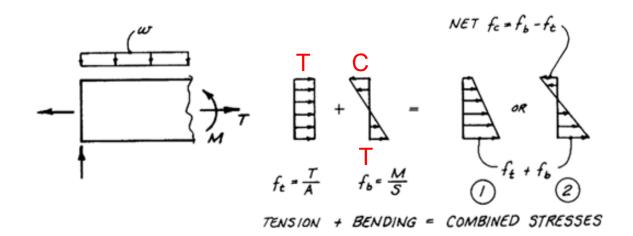
$$M = PL/4 = 0.15*12/4 = 0.45 in-lbs$$

$$fb = M/Sy = 0.45/0.0003255 = 1382.49 psi$$

 Next apply an additional axial tension force to the stick of approximately 10 lb (pull on it) and note the change in deflection. Calculate the additional axial stress.

$$ft = P/A = 10/0.03125 = 320 psi$$

Make a sketch showing the addition of the stress profiles of flexure + tension.



Now apply (or try) an axial compression load of approximately 10 lb to the stick and again note the change in deflection. Again calculate the axial stress.

$$fc = P/A = 10/0.03125 = 320 psi$$

Make a sketch showing the addition of the stress profiles of flexure + compression.

7. What additional load and stress is being neglected in the case of compression + flexure?

Second Order Stress "P Delta Effect"

With larger deflections this can become significant.

- 1. Eccentric load causes bending moment
- 2. Bending moment causes deflection,  $\Delta$
- 3.  $P \times \Delta$  causes additional moment



Any Questions?

yifanma@umich.edu

Thank You!

