

# Recitation 13

Combined Stress

# Homework problem

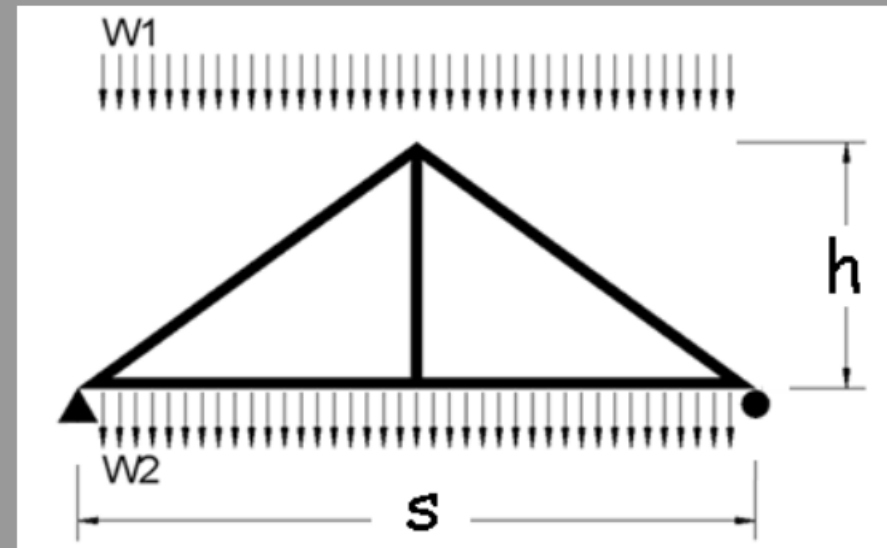
Combined Stress

## 12. Combined Stress

The given roof truss carries both an exterior snow load and an interior (attic) floor load. Determine the member forces and stresses and calculate the combined stress levels (top and bottom) for the lower chord member using the NDS combined stress equations. Consider all joints pinned, with simple (joint to joint) members. The given allowable stresses ( $F'_t$  and  $F'_b$ ) are for southern pine with all adjustment factors already applied.

DATASET: 1     

Full span of truss	26 FT
Height of truss	9 FT
On Center spacing of trusses	16 IN
Size of bottom chord	4x6
Actual width, $b$	3.5 IN
Actual depth, $d$	5.5 IN
Snow Load on roof, $w_1$	20 PSF
Live Load in attic, $w_2$	60 PSF
Factored allowable bending stress, $F'_b$	1150 PSI
Factored allowable tension stress, $F'_t$	690 PSI



Q<sub>1</sub>) Load on one truss - top chord (w<sub>1</sub>):-

$$\text{From } w_1 \text{ (PLF)} = w_1 \text{ (PSF)} \times \text{O.C. spacing}$$

$$= 20 \frac{\text{lb}}{\text{ft}^2} \times 16 \text{ in} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

$$\boxed{= 26.667 \text{ lb/ft}}$$

Q<sub>2</sub>) Load on one truss - bottom chord (w<sub>2</sub>):-

$$\text{from } w_2 \text{ (PLF)} = w_2 \text{ (PSF)} \times \text{O.C. spacing.}$$

$$= 60 \frac{\text{lb}}{\text{ft}^2} \times 16 \text{ in} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

$$\boxed{= 80 \text{ lb/ft}}$$

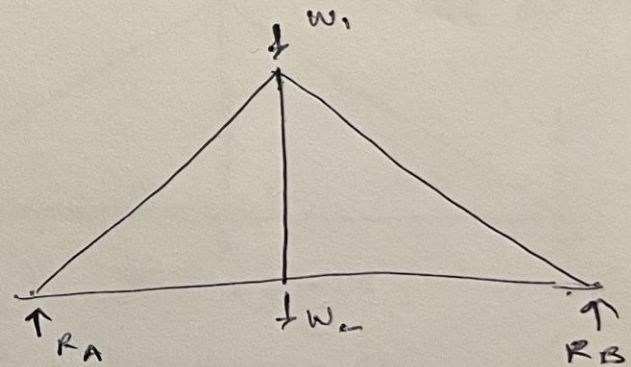
Q<sub>3</sub>) Total left reaction due to w<sub>1</sub> and w<sub>2</sub> :-

Pointed load (w<sub>1</sub>):-

$$= w_1 \text{ (PLF)} \times S \text{ (full span of truss)}$$

$$= 26.667 \times 26$$

$$= 693.342 \text{ lbs}$$





$$\begin{aligned} \text{Pointed Load } (W_2) &= w_2 (\text{PLF}) \times S \text{ (full span of truss)} \\ &= 80 \times 26 \\ &= 2080 \text{ lbs} \end{aligned}$$

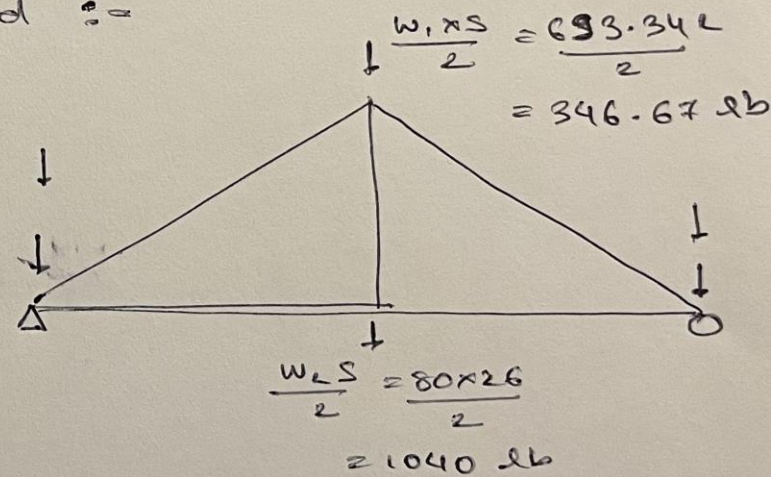
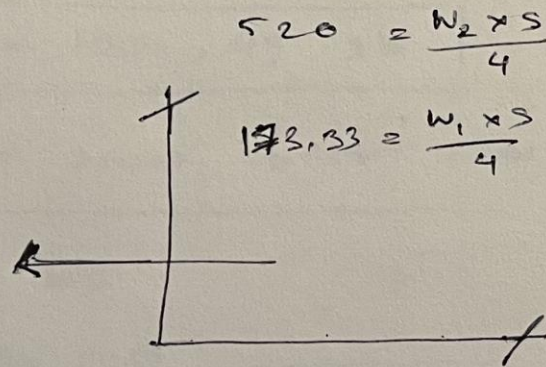
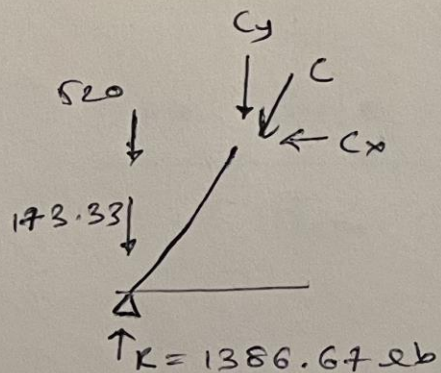
$$R_A = R_B$$

$$R_A + R_B = w_1 + w_2$$

$$R_A = \frac{w_1 + w_2}{2} = \frac{693.342 + 2080}{2}$$

$$R_A = R_B = 1386.671 \text{ lbs} \rightarrow \boxed{1386.671}$$

Q4) Vertical force component in truss top chord :-



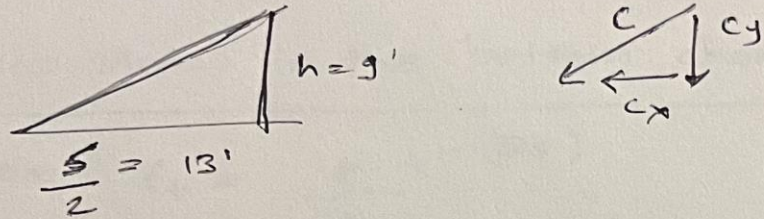
From  $\sum F_y = 0 \rightarrow 1386.67 - 520 - 173.33 - C_y = 0$



$$\therefore \boxed{C_y = 693.34 \text{ lb}}$$

Q5) Horizontal force component in truss top chord :-

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$$\frac{C_y}{C_x} = \frac{h}{s/2}$$

$$\frac{693.34}{C_x} = \frac{9}{13}$$

$$\boxed{C_x = 1001.49 \text{ lb}}$$

Q6) Axial force in the truss bottom chord :-

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From  $\sum F_x = 0$

$$T - C_x = 0$$

$$\boxed{T = 1001.49 \text{ lb}}$$



Q7) Area of the bottom chord member :-

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$$\begin{aligned}\text{from Area} &= b \times d \\ &= 3.5 \times 5.5\end{aligned}$$

$$\therefore \text{Area} = 19.25 \text{ in}^2$$

Q8) Axial stress in the bottom chord :-

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$$\text{from } f_t = \frac{P}{A} \leftarrow \text{(Q6)}$$

$$= \frac{1001.49 \text{ lb}}{19.25 \text{ in}^2}$$

$$\boxed{f_t = 52.025 \text{ lb/in}^2}$$

Q9) Maximum bending moment in the bottom chord :-

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$$\text{from } M_{\text{max}} = \frac{wL^2}{8} = \frac{wL \left(\frac{L}{2}\right)^2}{8}$$

$$= \frac{80 \times 13^2}{8}$$

$$= \boxed{1690 \text{ lb.ft}}$$



Q10) Section modulus of bottom chord member ( $S_x$ ):-

$$\text{From } S_x = \frac{I}{c} = \frac{\frac{1}{12} bd^3}{d/2}$$

$$= \frac{bd^2}{6}$$

$$S_x = \frac{3.5 \times 5.5^2}{6} = 17.646 \text{ in}^3$$

Q11) Maximum bending stress in the bottom chord member :-

$$\text{From } f_b = \frac{M}{S_x} = \frac{1690 \text{ lb}\cdot\text{ft}}{17.646 \text{ in}^3} \times \frac{12 \text{ in}}{1 \text{ ft}}$$

$$\therefore f_b = 1149.27 \text{ lb/in}^2$$

Q12) Combined stress using ASD equation 3.9-1 :-

from 3.9.1

$$\frac{f_t}{F_t'} + \frac{f_b}{F_b'} = \frac{5.2 \cdot 0.25}{690} + \frac{1149.27}{1150}$$

$$= 1.075$$

1.075 > 1.0 ∴ fail.

ⓧ

write the value as answer for Q12



Q13) Combined stress using MSD equation 3.9-2 :-

$$\frac{f_b - f_t}{F_b^{**}} = \frac{1149.27 - 52.025}{1150}$$

$$= 0.954$$

$$0.954 < 1.0 \quad \therefore \text{pass}$$

Q14) Does member pass or fail?

since 3.9-2 equation value is greater than 1.0

$\therefore$  member fails =  $\boxed{0}$

## Combined Stress

### Description

This project uses observation of a physical trial to see the effects of flexure combined with tension or compression.

### Goals

- To observe the behavior of tension + flexure
- To observe the behavior of compression + flexure
- To estimate the addition of combined stress profiles
- To observe results of P + delta loading

### Procedure

1. Load the 12 inch wood stick with 4 washers at midspan as shown below. The stick is 1/16"x1/2" A=0.03125 in<sup>2</sup> S<sub>y</sub>=0.0003255 in<sup>3</sup> 4 washers = 0.15 lbs.
2. Note the deflection caused by the load. Calculate the flexure stress.
3. Next apply an additional axial tension force to the stick of approximately 10 lb (pull on it) and note the change in deflection. Calculate the additional axial stress.
4. Make a sketch showing the addition of the stress profiles of flexure + tension.
5. Now apply (or try) an axial compression load of approximately 10 lb to the stick and again note the change in deflection. Again calculate the axial stress.
6. Make a sketch showing the addition of the stress profiles of flexure + compression.
7. What additional load and stress is being neglected in the case of compression + flexure?



$$M = \frac{P L}{4} \quad f_b = \frac{M}{S_y} \quad f_t = \frac{P}{A} \quad f_c = \frac{P}{A} \quad f_{comb} = \pm \frac{M}{S_y} \pm \frac{P}{A}$$



LAB

$$\tau_A = \frac{PL}{4}$$

$$\tau_A = \frac{0.15(12)}{4}$$

$$= 0.45 \text{ inch point.} \quad \textcircled{1}$$

$$f_b = \tau_A / s_y$$

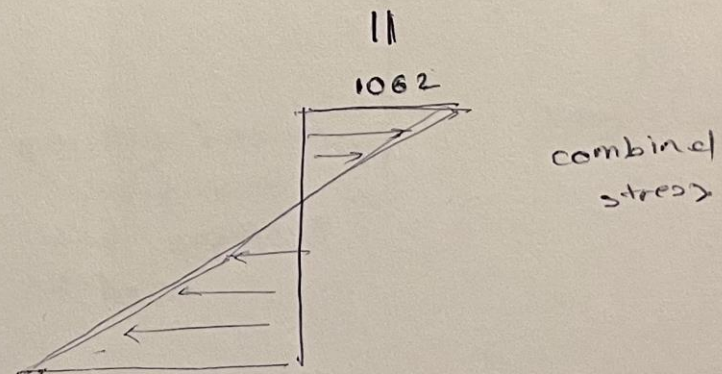
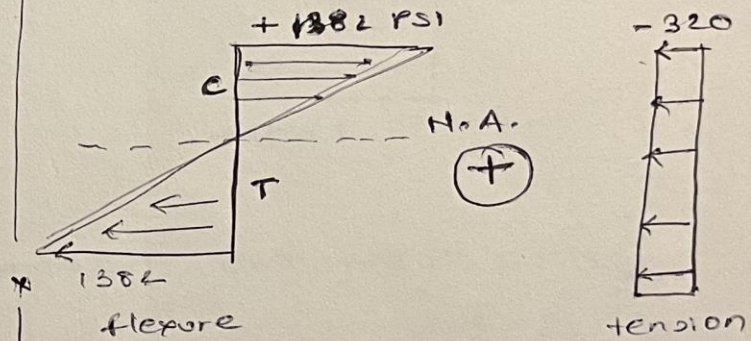
$$= \frac{0.45}{0.0003255}$$

$$= 1382 \text{ psi} \quad \textcircled{2}$$

$$f_t = \frac{10}{0.03125} \quad \leftarrow \text{axial tension force}$$

$$= 320 \text{ psi} \quad \textcircled{3}$$

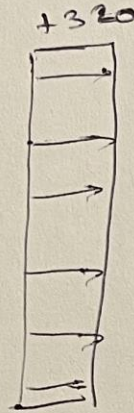
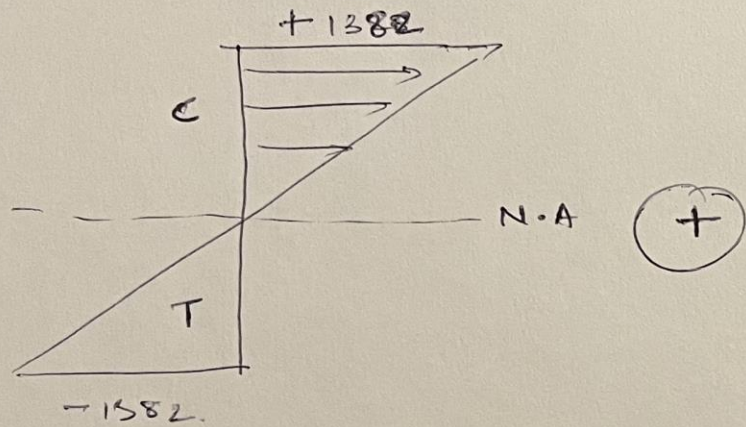
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Graph for bending stress and tensile stress :- (and two together) :-



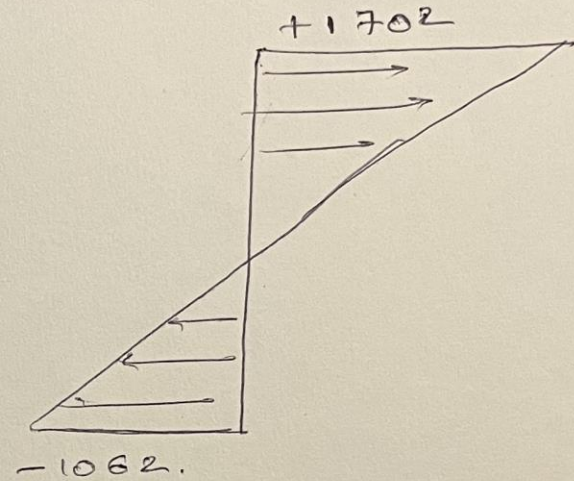


$$f_c = \frac{10}{0.03125}$$

2320 psi ——— (5)



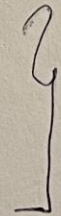
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(7) Additional stress caused by AP moment over weak axis.

$$M_{AP} = P \times \text{deflection}$$

$$= 10 \times \begin{matrix} (1) & = & 10 \\ (2) & = & 20 \\ (3) & = & 30 \\ (4) & = & 40 \end{matrix}$$



quickly becomes more than the capacity of beam

$$\frac{d}{r} = 192 > 50$$

↓  
very slender.



Thankyou !!!